### Synthesized earthquake ground motions for earthquake resistant design

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ABSTRACT: Some historical reviews on the past research works and achievements on synthesizing earthquake ground motions were first introduced with comments. Based upon the above reviews, it has been found that one of the practical procedures to generate synthetic earthquake ground motions is the one by superposition of sinusoidal waves with a set of phase angles of real accelerograms. The set of phases of real accelerograms gives synthetic waves the informations on nonstationarity corresponding to envelope functions. This discovery led to the technology to apply the specific unique functions on phases. Then was presented the synthetic ground motion combined seismological approach by fault model with the sinusoidal superposition procedure.

### 1 INTRODUCTION

In the earthquake resistant design of structures, if unlimited data of recorded earthquake ground motions (recorded EGMs) were available, representative stochastic models could be established directly by statistical analyses based upon those data. Unfortunately recorded strong EGMs are rather limited. Therefore one is forced to hypothesize a form of model of EGM for seismic design by generating synthetic earthquake ground motions (synthetic EGMs) on the basis of statistical characteristics of recorded EGMs, such as averaged power spectra, response spectra and so on.

In earlier studies the EGMs were synthesized by the procedure through which the white noise was first generated and filtered, or the EGM was directly synthesized under the specific power spectrum, then the envelope function was multiplied. In the next days, mainly for the purpose of seismic design of nuclear power plants, the methods to synthesize EGM appropriate to the design response spectrum (DRS) were developed. Those researches are first summarized

that the phases of recorded EGMs are strongly correlated with the envelope functions of EGMs. Based upon this fact, the procedure to synthesize EGM by actual set of phases of recorded EGMs was proposed. The unique method was also proposed, by which synthetic EGM can be generated so

as to fit DRSs simultaneously with two kinds of critical damping ratios.

#### 2 SYNTHETIC STATIONARY WHITE NOISE

EGMs were first idealized into the stochastic model with random pulses by Housner (Ref.1). A series of impulses, Gaussian shot noise or white noise were utilized to synthesize EGMs by several researchers in those earlier days.

Housner (Ref.1) and Rosenblueth (Refs.2,9) idealized EGMs by a series of impulse randomly distributed in time space as shown in Fig.1. The acceleration a(t) is represented as

$$a(t) = \sum V_i \cdot \delta(t-t_i) \qquad (1)$$

in which v denotes the impulse magnitude and  $\delta(t)$  is the Dirac delta function. The auto-correlation function  $R(\tau)$  in terms of only time difference  $\tau$  and the power spectral density function  $S(\omega)$ , of a(t) are

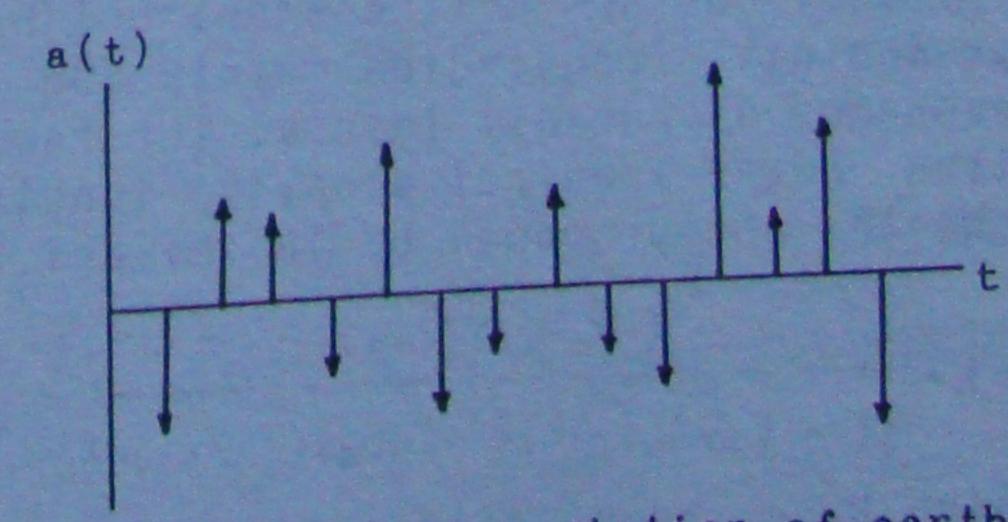


Fig.1 Impulse representation of earthquake

then obtained as follows.

$$R(\tau) = \sum_{i=1}^{\infty} E[V_i^2] \cdot \delta(\tau)$$
 (3)

$$S(\omega) = \frac{\tau}{2^{1/2}\pi \cdot E[V_i^2]} = const.$$

Therefore the synthetic EGM expressed by Eq.(1) has the constant amplitudes in all frequencies, the synthetic EGM of this type

is well known as white noise. The white noise was synthesized by the use of electrical simulator by Bycroft (Ref.4) and Naka et al. (Ref.12), or an electric analog computer by Ward (ref.13). The velocity response spectra excited by white noise (Bycroft) are shown in circles of Fig. 2, in comparison with Housner's response spectra (Ref.3) derived

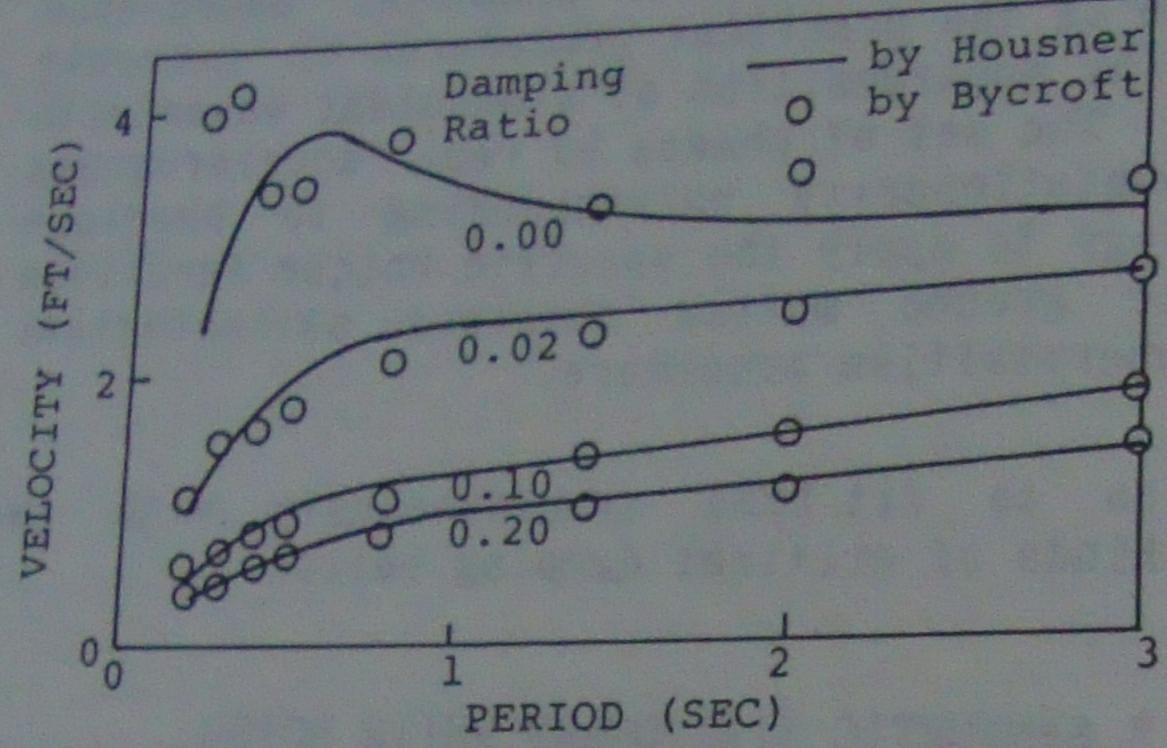


Fig. 2 Housner's averaged response spectra and "white noise" spectra by Bycroft

from eight recorded strong EGMs. In the Bycroft's study, the velocity response spectra subjected to white noise with duration time of 25 seconds tend to be constant in the longer periods. As the result, the representation of EGMs by white noise having finite duration time seems to be reasonable at the first stage of the synthesizing. Then Ruiz and Penzien (Ref. 18) generated white noise by the use of a function in terms of a pair of random variables.

# 3 FREQUENCY CONTENTS OF EARTHQUAKE GROUND

In the recorded EGMs, the power spectral density functions are not constant but have the predominat frequencies mainly due to the resonance of subsoil layers. The representative power spectral density function might be the one proposed by Tajimi(Ref.7);

$$S(\omega) = K \cdot \frac{1 + 4 \, h_g^2(\omega/\omega_g)^2}{\{1 - (\omega/\omega_g)^2\}^2 + 4 h_g^2(\omega/\omega_g)^2}$$
in which K is the magnitude of the spec-

corresponds to the predominant of the expresses frequency of EGM and h expresses ratio of subsoil layers. The trum, wo frequency of Eum and I subsoil matic shapes of power spectra proposed the matic shapes (Refs. 5-7, 19-22) matic shapes (Refs.5-7,19-22) are illing

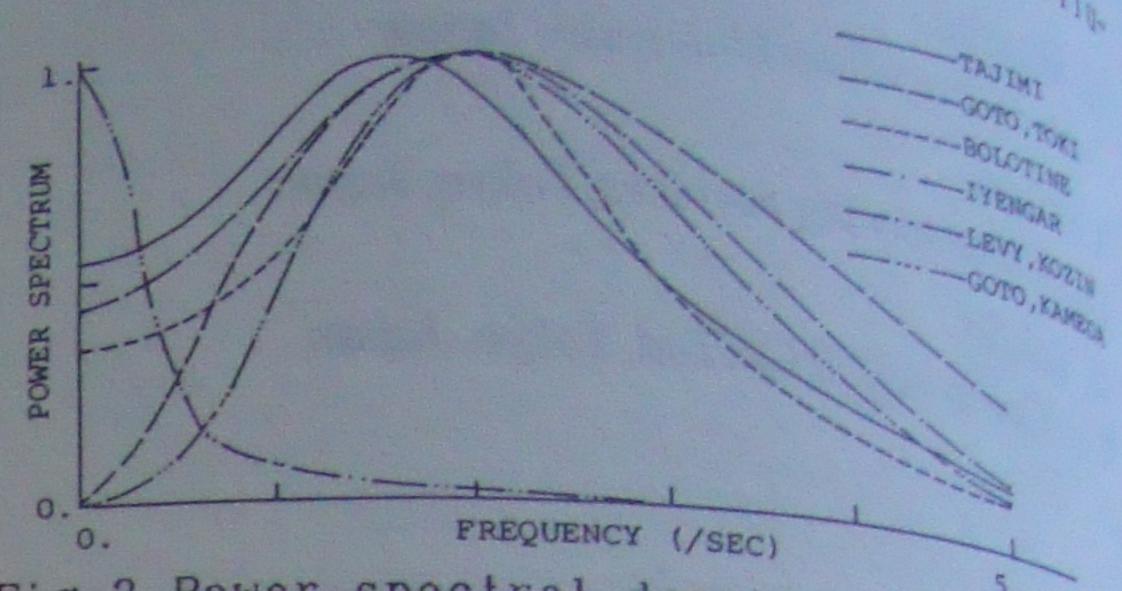


Fig.3 Power spectral density function

the synthesizing of EGM with specific frequency contents, two methods have been found. One is based on the filtered white noise as expressed by Lin (Ref.10) and in the other method EGMs are directly synthesized by the superposition of sinusoidal waves. The power spectrum is once defined by Eq. (4), the synthesizing was carried out through the filter of Eq.(5) by Housner and

$$\ddot{z}(t) + 2h_g \omega_g \dot{z}(t) + \omega_g^2 z(t) = -\sqrt{K} \cdot N(t)$$
 (5)

Where N(t) is the white noise with unit power spectrum.

The direct method by the superposition of sinusoidal waves is generally expressed by either Eq.(6) proposed by Toki (Ref.17) or Eq. (7) by Shinozuka and Jan (Ref. 25).

$$a(t) = \sigma_y \cdot \sum_{i} COS(\omega_i t + \phi_i)$$

in which  $\sigma_y^2$  is the variance of EGM, andw and p has the probabilistic density function of  $S(\omega)/\sigma_y^2$  and uniform density, respectively.

$$a(t) = \sum_{j} A_{j} \cdot COS(\omega_{j}t + \phi_{j})$$

$$A_{j} = \sqrt{4S(\omega_{j})} \cdot \Delta \omega$$
(7)

where  $\omega$  is the deterministic frequency equally spaced by  $\Delta \omega$  and  $\phi$ is with random variables for phases.

### 4 NONSTATIONARY EARTHQUAKE GROUND MOTIONS

The recorded EGMs are composed of the three parts of envelope in time space; build-up nearly constant with high intensity level and decaying (Ref. 15). The simple model of nonstationary synthetic EGM is expressed as

follows.

$$a(t) = I(t) \cdot b(t)$$
 (8)

where b(t) is the stationary random process synthesized by the method as described in synthesized and I(t) is the envelope function. Sec. 3 and I(t) becomes the variance of a(t) becomes

$$E[a^{2}(t)] = I^{2}(t) \cdot E[b^{2}(t)]$$
 (9)

where E[b²(t)] is independent of time, therefore the change of intensity of a(t) in terms of time can be given by only I(t). In terms of time can be given by only I(t). Derived from these properties I(t) is also called the deterministic intensity function. Fig. 4 shows the envelope functions proposed by some researchers proposed (Refs. 5, 14, 15, 19, 21) and the similar functions are proposed by the others (Refs. 8, 16, 24).

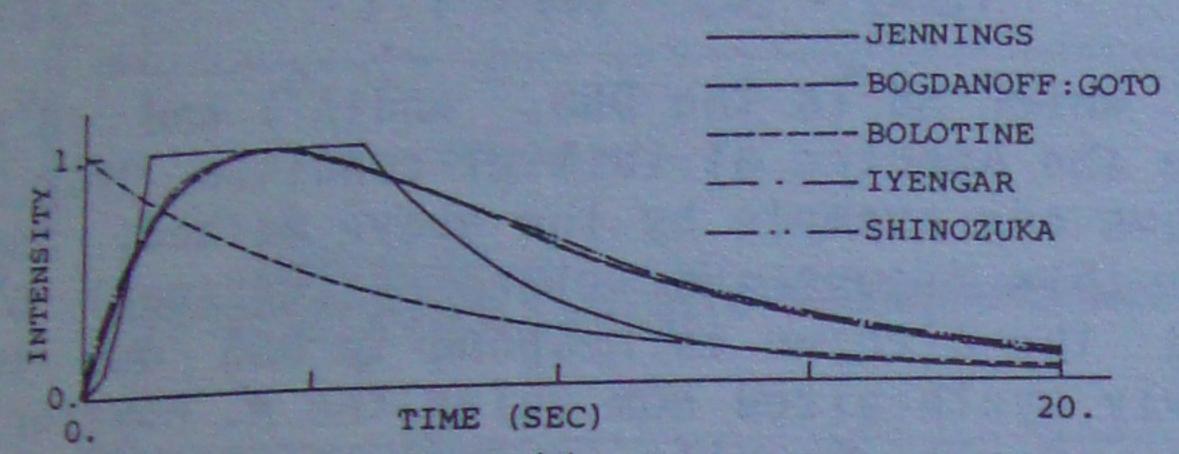


Fig. 4 Envelope function

The above description was limited to the nonstationarity of intensity level in time domain. The nonstationarity of recorded EGMs has been found in frequency contents with respect to time as well. As for the synthetic EGM having the nonstationarity in both intensity and frequency, Saragoni and Hart (Ref.28) tried to synthesize EGMs by partitioning EGMs into some parts with different frequency characteristics in time space. Hoshiya and Isoshima (Ref. 32) and Kameda and Sugito (Ref. 33) proposed the synthetic methods by the use of similar expression of Eq. (7), in which the power specta are dependent on time. In these methods Hoshiya used the physical spectrum concepts and Kameda the evolutionary power spectrum.

### 5 SYNTHETIC EARTHQUAKE GROUND MOTIONS AP-PROPRIATE TO THE DESIGN RESPONSE SPECTRA

For the earthquake resistant design of structures, the response spectra are the principal factors on EGMs. From this point of view many investigations on the DRS derived from the recorded EGMs have been performed and some of them (Refs. 38, 39) have been often utilized for seismic design of nuclear power plants. When the DRS are

given as the spectral characteristics of synthetic EGM, it is necessary to generate EGM appropriate to the DRS.

The earlier research in this direction was presented by Watabe (Ref.23). In this study the EGM was generated through an iterative procedure focussing on the preestablished spectrum G. Using the spectrum Gc calculated by synthetic EGM, the pre-set amplitude A expressed in Eq.(7) was transformed into newly set amplitude A at the next iterative step as follows.

$$A'(\omega) = A(\omega) \cdot G(\omega) / Gc(\omega) \qquad (10)$$

Most of the later researches had the similar procedure to fit the DRS.

Tsai (Ref.26) and Rizzo et al. (Ref.27) obtained the synthetic EGM appropriate to the DRS Sd by modifying a recorded EGM. For the iterative procedure, Tsai used suppressing technique passing through an transfer function if the response spectrum subjected to synthetic EGM (RSS) Sc>Sd, and the raising technique by adding a sinusoidal wave if Sc(Sd. Rizzo et al. carried out the procedure by the same way of Eq. (10) if Sc>Sd and the similar way by Tsai if Sc(Sd. the method by Levy and Wilkinson (Ref. 30), the amplitudes were modified by Eq. (10) and a sequence of 0 and  $\pi$  was selected for the phases expressed in Eq. (7).

The method for directly setting amplitude spectrum of the synthetic EGM modelled by Eqs. (7) and (8) was proposed by Vanmarcke and Gasparini (Ref.31) and Iyengar and Rao (Ref. 34), by which synthetic EGM can be generated so as to fit the DRS without iteration. In the proposal by Vanmarcke et al. the relationships between amplitude spectra and response spectra based on the random vibration theory were applied. It is herein noted that the synthetic EGM by Vanmarcke is appropriate to the DRS in a stochastic sense such as ensemble average. Iyengar et al. set up the equations on the response time histories subjected to a synthetic EGM, and estimated the amplitude spectrum composed of 25 frequency components. Then their method was extended so as to satisfy simultaneously the DRSs with two different damping ratios. From their examples the fitness of spectra to DRS at the 1st iteration seems not to be improved.

6 SOME DISCUSSIONS ON THE ITERATIVE WAY TO SYNTHESIZE EARTHQUAKE GROUND MOTIONS FITTING THE DESIGN RESPONSE SPECTRA

The aspect of change of RSS due to the modification of amplitudes  $A(\omega)$  of synthe-

tic EGM in Eq. (7) has been studied by The EGM was generated by the model of authors as follows.

Eq.(8), using the envelope function by Jennings et al. with the duration time of 30sec. As the spectral properties defined by Eq.(4), K=225gal \*sec, ωg=2π/0.4rad /sec., and h = 0.6 were assumed. At first, the response spectrum (So) at the same frequencies  $\omega_i(=i \cdot \Delta \omega; \Delta \omega = 1.25 \text{ rad/sec.}, i = 1-$ 50) of sinusoidal frequency components in Eq.(7) was calculated by the synthetic using the initial amplitudes {A°} Eq.(7). Next an amplitude α(Gal) at a frequency  $\omega_i$  was added to  $A_i^0$ , then new response spectrum (Sj) subjected to the new synthetic EGM was computed. Using these results, the coefficients of influence Cij were defined as follows.

$$C_{ij} = \{S_v^{\dot{a}}(\omega_i) - S_v^{\dot{a}}(\omega_i)\}/\alpha$$
 (11)

Figure 5 shows C in three conditions  $(\alpha = 1, \alpha = 10, \alpha = 1)$ and Fig.6 shows  $C_{ij}$  in  $\alpha = 1$ . The results suggest that the increase of Sv at the frequency of resonance is roughly proportional to an additional amplitude, therefore the iterative procedure by Eq. (10) was affirmed to be one of the methods to success the synthesizing of EGM appropriate to DRS.

As for that purpose, it might be expected to utilize the matrix [C] composed of the coefficients of influence. The method can

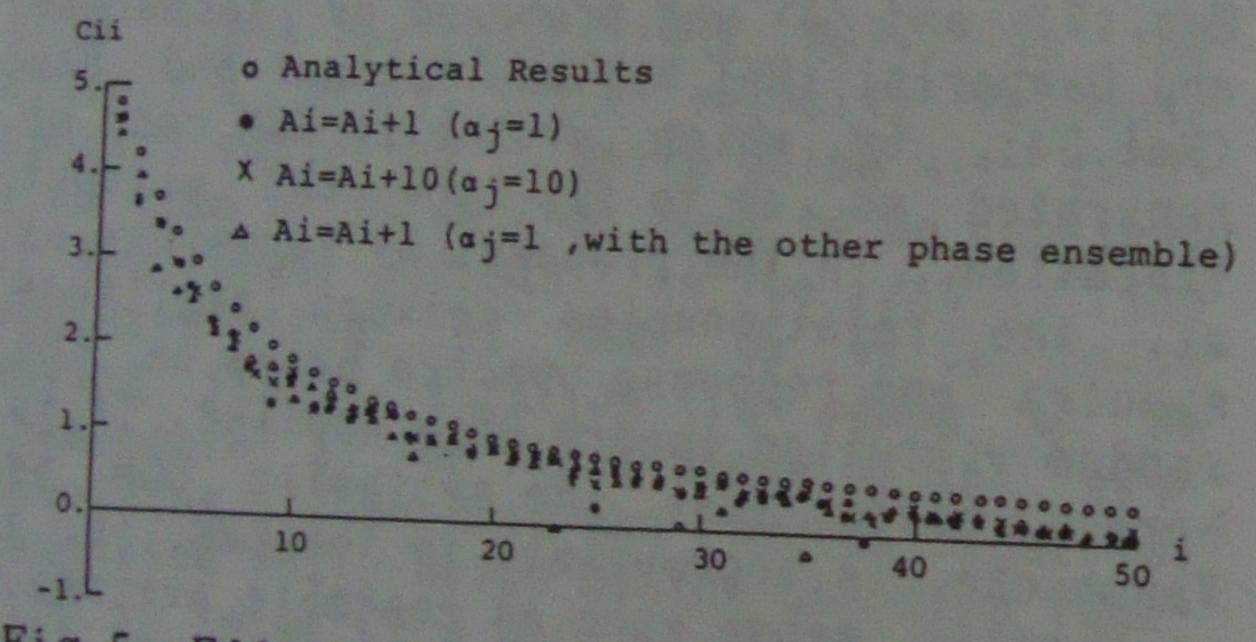
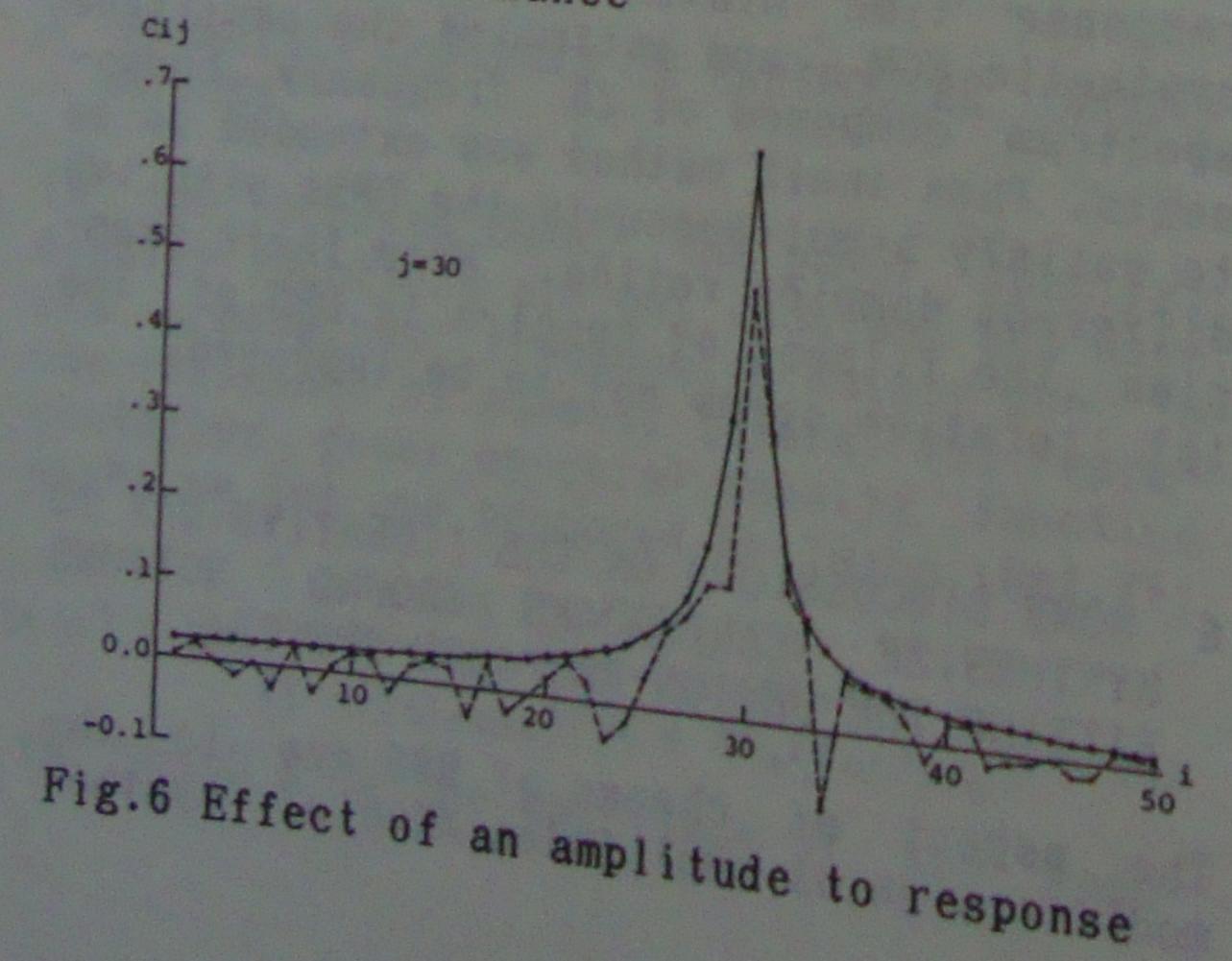


Fig.5 Effect of an added amplitude to response in resonance



M. 7.00 EP1 - 20.0 A Initial value O Results used influnce matrix

Fig.7 Comparison between DRS and RSS due to

be written in the followings.

$$\{A\}' = \{A^{\circ}\} + \{\Delta A\}$$
  
 $\{\Delta A\} = [C]^{-1} \cdot \{S_{d} - S^{\circ}\}$ 

in which Sd is the DRS, and {A<sup>0</sup>} and {S<sup>0</sup>} are the spectra at initial condition. Fig. shows one example by the above method, and in this figure the triangles represent and the circles correspond to RSS due to newly synthesized EGM using the M. However, the example results in that the fitness is not acceptable, because the relationships between the increase of amplitudes and RSS are not linear as to be inferred by the difference of Cii in Fig.6 and the occurerce time of peak response changes in each synthesizing.

Incidentally the criterion of fitness in the following to judge the completion of iteration was proposed by the authors, by which the responses of structures less variate in spite of the procedure to synthesize or the selection of phases in Eq. (7).

in which  $\varepsilon(Ti) = Sc(Ti)/Sd(Ti)$  and Sc and Sare the RSS and DRS, respectively, and i and v are the mean value and the coeffi cient of variation of  $\varepsilon(Ti)$ .

7 CHARACTERISTICS MOTIONS AND THEIR EARTHQUAKE GROUND APPLICATIONS

Most of the researches on the field synthetic EGM suggested the randomness and the uniform probability distribution, phases in EGM. It is also the fact, however that EGM with nonstationarity is not posed only of Fourier amplitudes but phase and phase angles. If only phases of a recorded

EGM are transformed to the random phase angles having the uniform probability density and the time history is generated, the time trace should become stationary random process different from the recorded one. Therefore using a set of phases of a recorded EGM, independent of amplitude spectrum ded EGM, independent of amplitude spectrum be the one of the procedures to find out the role of phases on nonstationarity of EGMs.

Fig. 8 shows the recorded EGM of Taft 1952, EW component, and synthetic one which has uniform amplitudes in each frequency component with the same set of phases of the Taft 1952. Let this kind of synthetic the Taft 1952. Let this kind of synthetic EGM be called as "phase wave". The phase wave is generated by the following way.

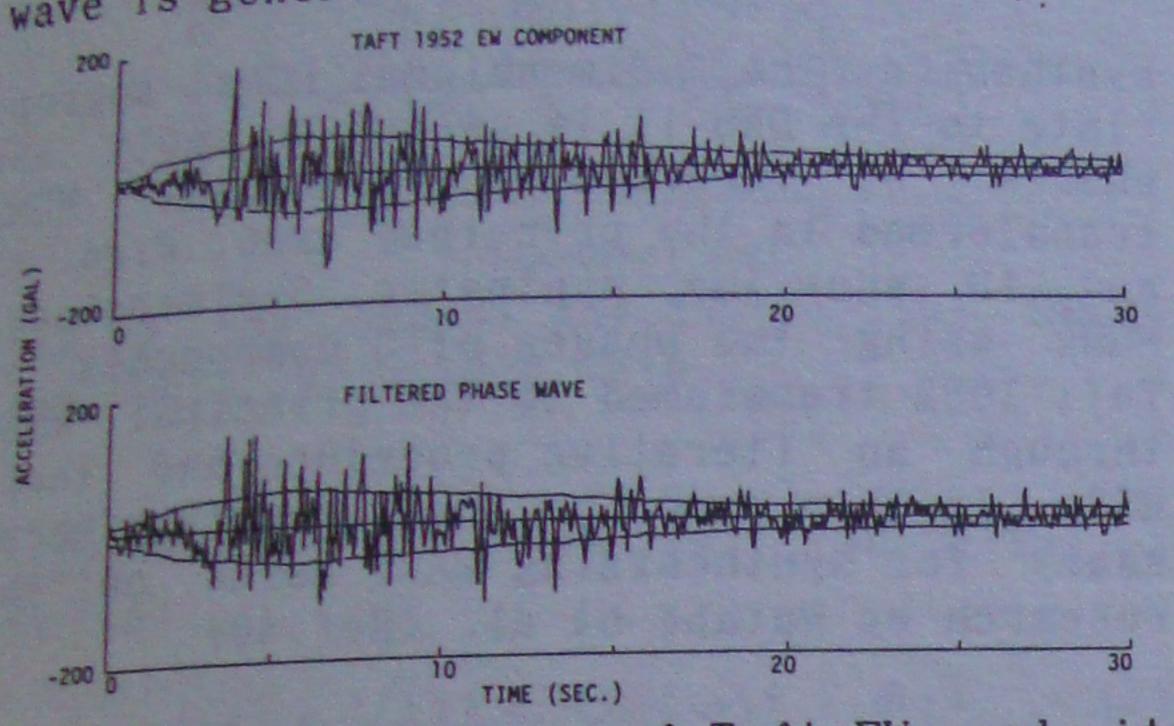


Fig.8 Original wave of Taft-EW and its phase wave

A digitized EGM a (t) can be expanded into a Fourier series as follows.

$$a_r(t) = \sum A_j \cdot COS(\omega_j t + \phi_j)$$
 (13)

The phase wave a (t) in Fig. 8 is generated by using the relation

$$a_p(t) = P \cdot \Sigma COS(\omega_j t + \phi_j)$$
 (14)

where P is the scaling factor of intensity and the high frequency components in a (t) are excluded in a (t). It is clearly shown in Fig.8 that the envelope function of phase wave is quite similar to the original one.

It was pointed out by Ohsaki (Ref. 36) that the characteristics of nonstationarity of recorded EGMs can be expressed by the distribution of phase difference  $\Delta \phi$ , that is given by

$$\Delta \phi_j = \phi_{j+1} - \phi_j \qquad (15)$$

where  $\Delta \phi$  is defined in the range  $-2\pi < \Delta \phi$  (0. Fig.9 shows the probability density functions of phases and phase differences of Taft 1952-EW component in the duration of 30 sec. respectively. It is seen from

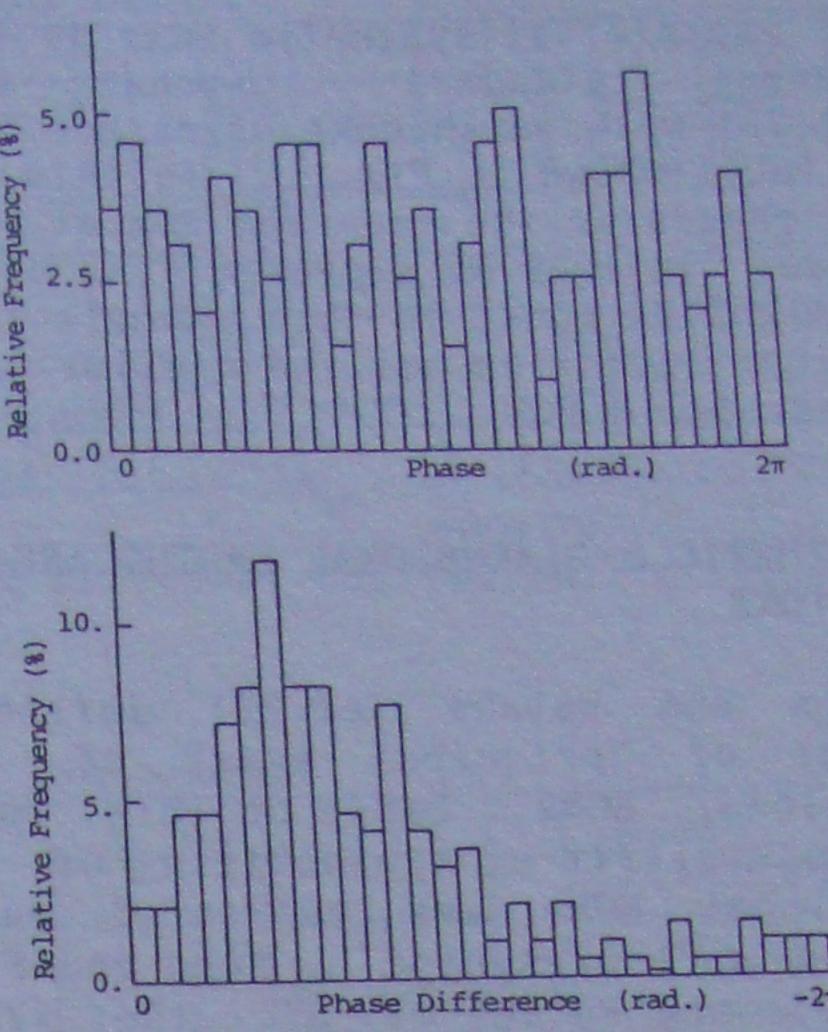


Fig.9 Probability density of phase (upper) and phase difference (lower) of Taft-EW

these figures that the probability density of phase angles is nearly uniform, while the probability density of phase differences has some characteristics similar to that of Normal-like. In addition as pointed out by Ohsaki, the distribution of phase differences normalized by  $-2\pi$  has a quite similar feature to the envelope function of time history normalized by the duration time.

The results discussed above suggest that it may be wise to utilize the properties of phases of recorded EGMs to controle envelope functions of synthetic EGMs. Fig. 10 is

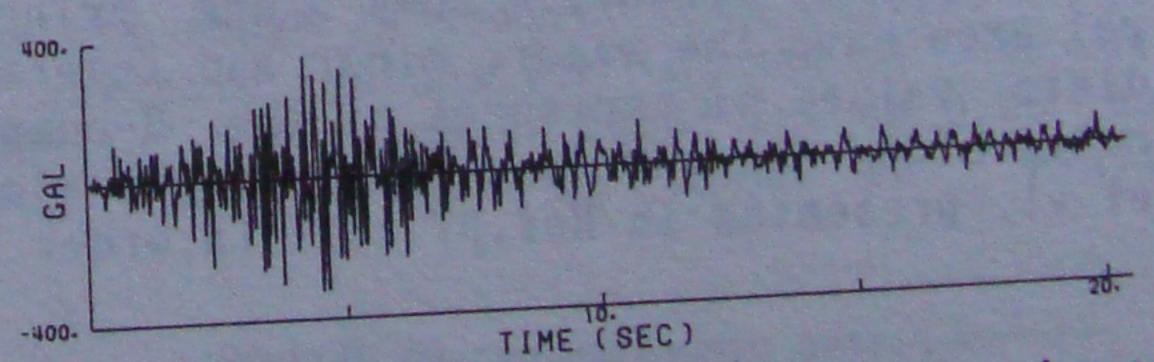


Fig. 10 Synthetic EGM using a set of phases of recorded Cholame Shandon-NS

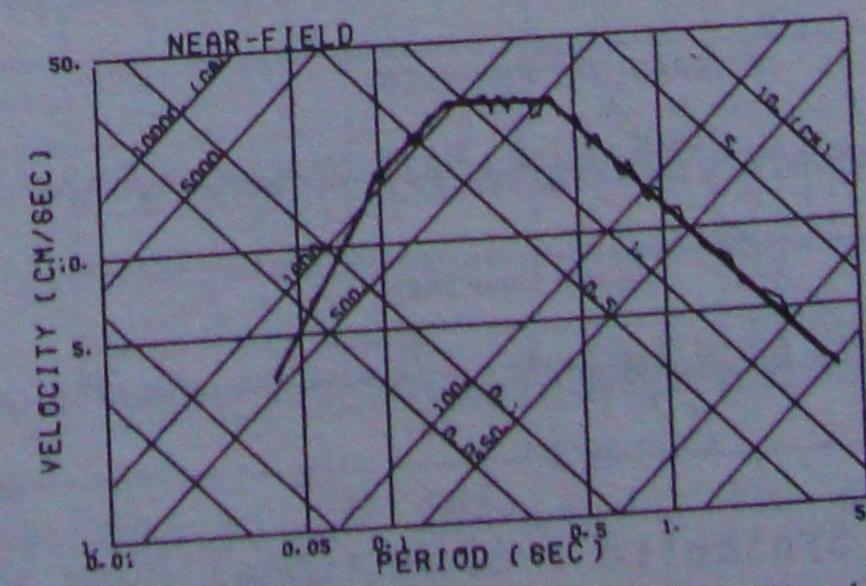


Fig.11 DRS and RSS due to synthetic EGM shown in Fig.10

one example of synthetic EGMs as to fit spectrum response standard response 6.5 near Japanese standard response 6.5 near (Ref.39) with earthquake magnitude example (Ref.39) with earthquake magnitude for this example the fault shown in Fig.11. In this cholame the fault shown in Fig.11. EGM at Cholame the phases of the recorded EGM at utilized. The shandon in 1966-NS component is utilized. It might be found in this example that the envelope function has the similar feature of near field EGM.

8 SYNTHETIC 3-DIMENSIONAL EARTHQUAKE GROUND MOTIONS

Penzien and Watabe (Ref.29) defined the concept of "principal axes" of multiconcept of "principal axes" of multidimensional EGMs. Based on this concept, the possibility of synthesizing of three-the pos

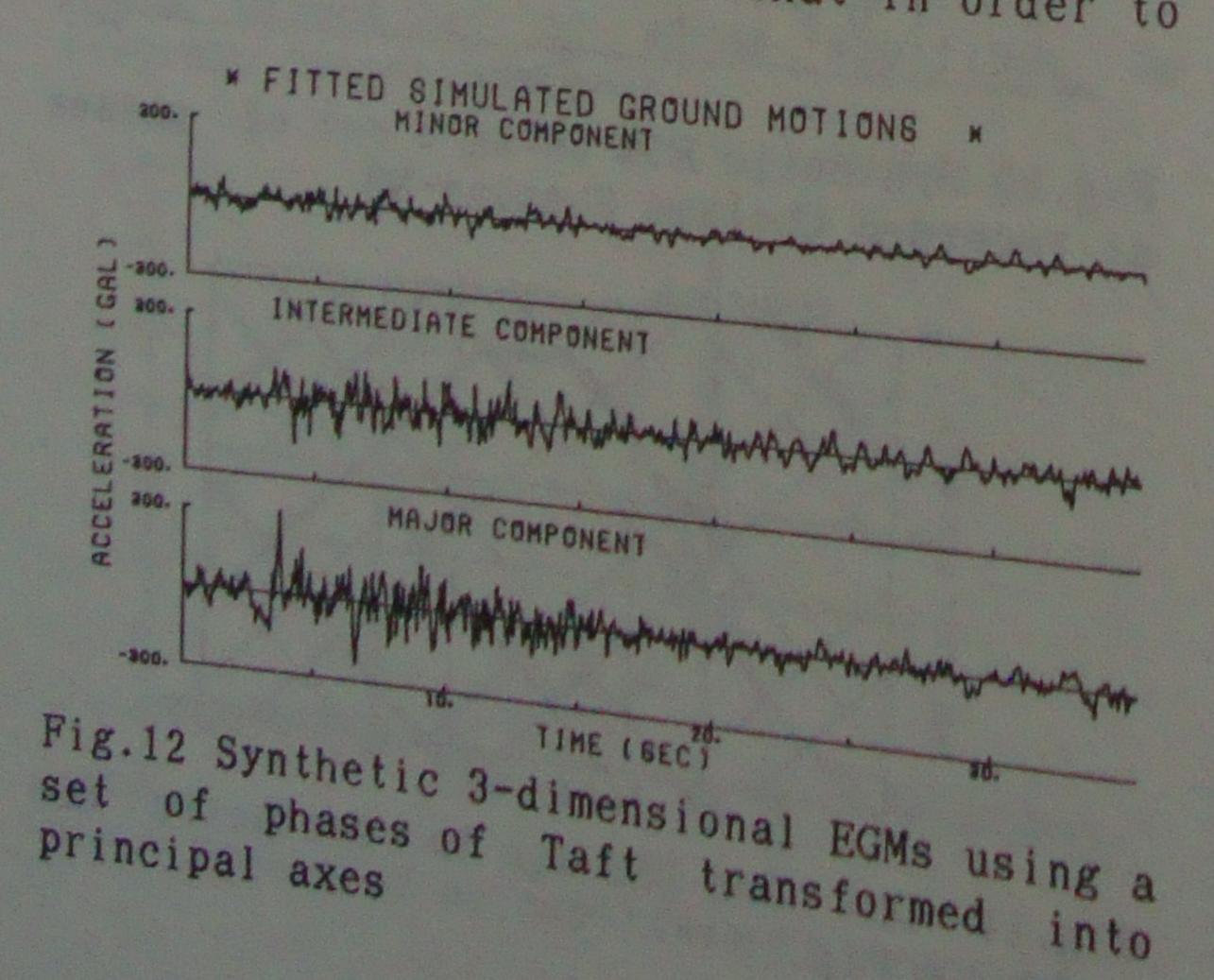
$$a_k(t) = I_k(t) \cdot b_k(t), k=x,y,z$$
 (16)

Where bx(t), by(t) and bz(t) are stationary random motions stochastically uncorrelated with each other, therefore the covariance matrix of components becomes the diagonal matrix as follows.

$$\underline{\mu(t)} = \begin{bmatrix} \mu_{xx} & \mu_{xy} & \mu_{xz} \\ \mu_{yx} & \mu_{yy} & \mu_{yz} \\ \mu_{zx} & \mu_{zy} & \mu_{zz} \end{bmatrix} = \begin{bmatrix} \mu_{xx} & 0 & 0 \\ 0 & \mu_{yy} & 0 \\ 0 & 0 & \mu_{zz} \end{bmatrix} (17)$$

$$\mu_{ij} = E[a_i(t)a_j(t)]$$

From the relation, it is apparent that the components of synthetic EGMs along principal axes have the major, minor and intermediate values of variances of the 3-components along the any axes tranformed. Watabe et al. presented in Ref.37 that in order to



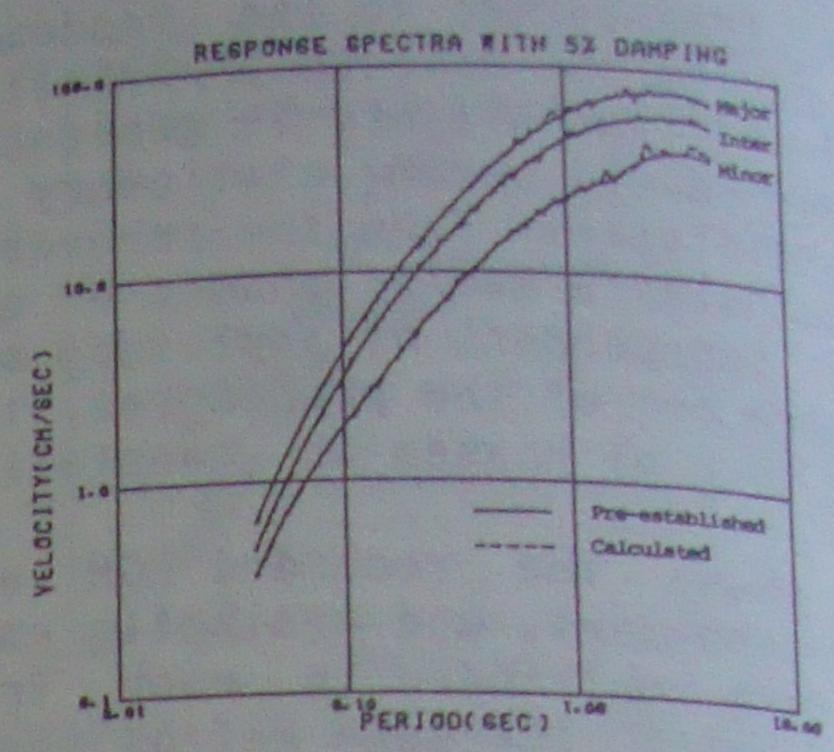


Fig. 13 DRSs and RSSs due to synthetic dimensional EGMs in Fig. 12

synthesize the 3-dimensional EGMs appropriate to the DRS it is useful to apply the phases of recorded 3-dimensional EGMs transformed to the principal axes. Figs. 12 and 13 show the synthetic 3-dimensional EGMs using the phases of 3 components of Taft 1952 transformed to the principal axes through an iterative procedure and their RSSs. In this example, the parameters necessary for synthesizing were based on the research by Watabe et al. (Ref. 40).

## 9 PHASE CHARACTERISTICS FOR TWO RESPONSE SPECTRA WITH DIFFERENT DAMPING RATIOS

In generating synthetic EGM with DRS, the iterative calculation is carried out to fit design response spectrum with 5% damping(DRS(5)). By this procedure, response spectrum with 5% damping subjected to synthetic EGM(RSS(5)) nearly converges to DRS(5). However RSS(1) due to the above synthetic EGM does not always converge to DRS(1) if uniform random numbers for phases are used, and this RSS(1) is generally larger than DRS(1). As one example, DRS(5) due to EGM recorded at Golden Gate is shown in solid line and RSS(5) in dotted line in Fig. 14(a) where the quite reasonable agreement between the two can be seen. Solid line in Fig. 14(b) is DRS(1), while dotted line in Fig.14(b) expresses RSS(1), may be judge to be over response than DRS(1) and irregular. This trend can be recognized in other EGMs. Plied to the structural system with posed of members with different damping ratios, it is desired that synthetic EGN should be in comformity with response spectra simultaneously with two different damp of about in performing seismic analysis of above structure systems. From this point of view, one idea to generate such synthe tic EGM is herein presented.

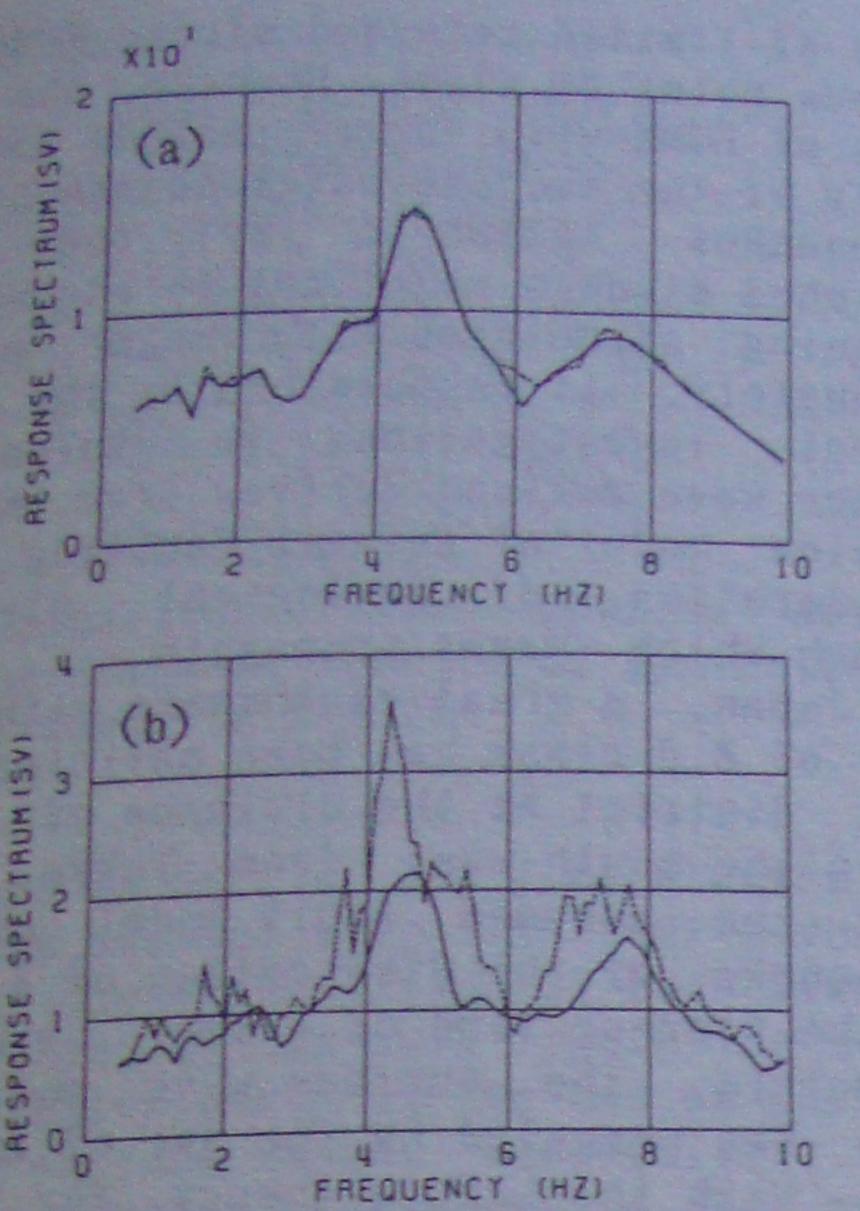


Fig.14 (a) DRS(5) and RSS(5), (b) DRS(1) and RSS(1) of Golden Gate

Synthetic EGM can be expressed in Eq.(18).

$$a(t) = I(t) \sum A_j \cdot COS(\omega_j t + \phi_j) \qquad (18)$$

Envelope function I(t) in Eq.(18) is not taken into account in the following discussion. Amplitude A is determined so as to fit RSS(5) to DRS(5). Then the remained freedom to control the other characteristics of synthetic EGM is only phase  $\phi$ .

Therefore, extensive researches on the role of phase angle have been conducted by the group of authors in order to realize the generation of synthetic EGM which satisfy the above requirement to fit the two DRS with different dampings. As the result the following function for phase of synthetic EGM is proposed.

$$\phi(f) = \alpha f + de^{-\chi} f - d + we^{-z} [-\pi, \pi]$$
 (19)

where  $[-\pi,\pi]$  is uniform random number between  $-\pi$  and  $\pi$  Coefficients  $\alpha$ , "d", "k", "w", and "z" are arbitrary constants. The term of coefficient  $\alpha$  in Eq. 19 corresponds to Fourier phase spectrum of an impulse wave over the duration and contributes to the shift of a wave form of synthetic EGM. " $\alpha$ " does not influence the maximum value of synthetic EGM, and still more the RSS(1). When coefficient "w" is assumed to zero, randomness element in Eq.(19) diminished, phase  $\phi$  reduces to only the exponential function.

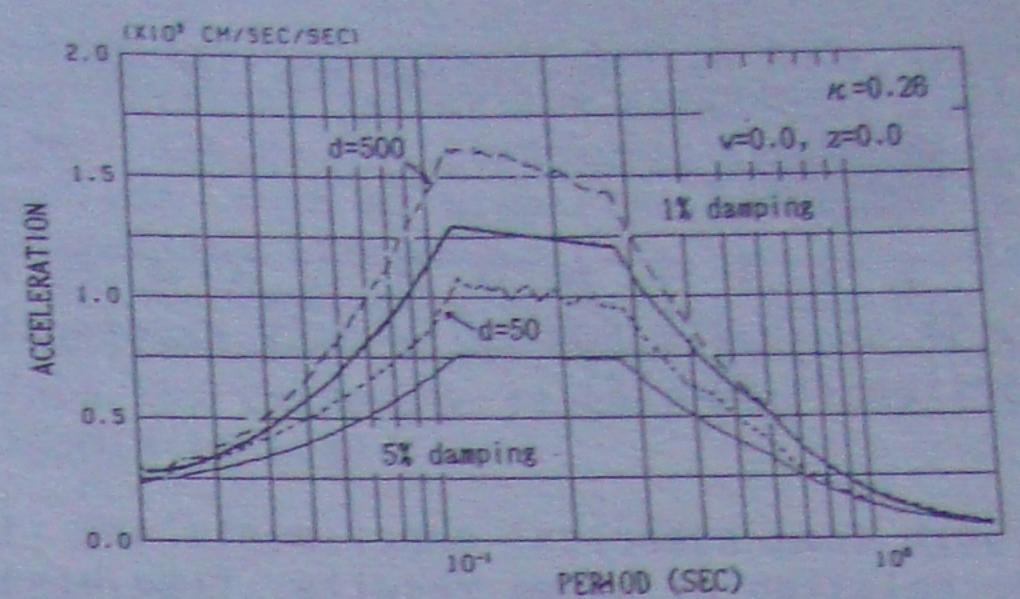


Fig. 15 Effect of coefficient "d" to RSS(1)

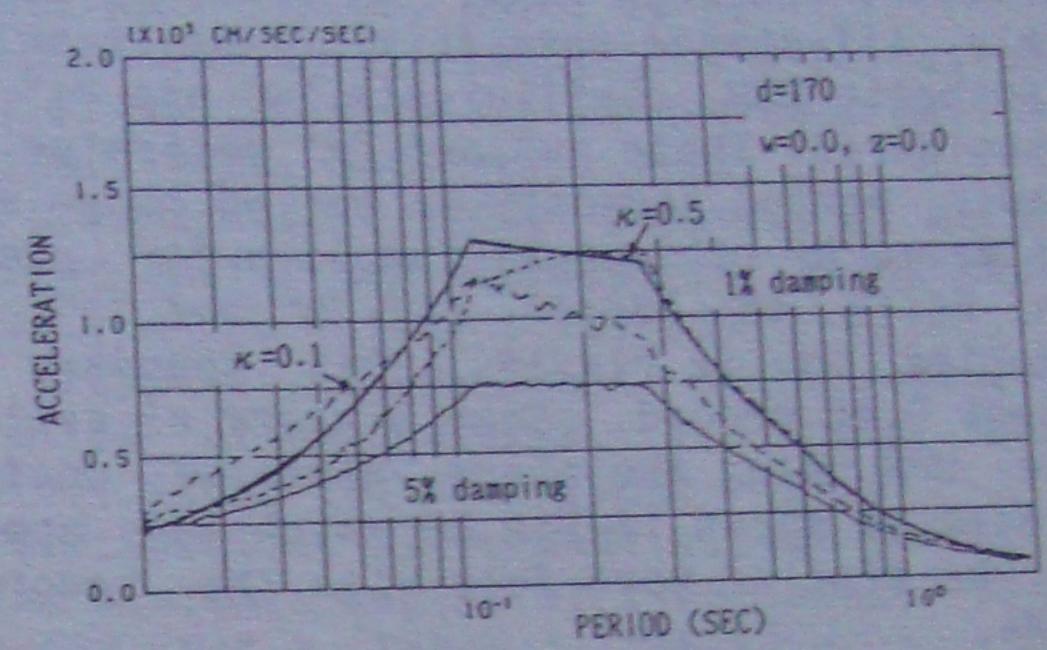


Fig. 16 Effect of coefficient "k" on RSS(1)

The effect to RSS(1) by variation of coefficient "d" is shown in Fig.15, in which the RSS(1) with d=50 becomes smaller than DRS(1) while the RSS(1) with d=500 larger.

The variation of coefficient " $\kappa$ " governes the inclination of rather flat part in RSS(1) in Fig.16. AS shown in Fig.16, when " $\kappa$ " is 0.5, the level of RSS(1) in shorter period range is smaller than DRS(1), while if " $\kappa$ " is 0.1 the level of RSS(1) in longer peiord range is smaller than DRS(1).

Suitable values for "k" and "d" to fit DRS(1) are finally found out as 0.26 and 170 respectively as shown in Fig.17

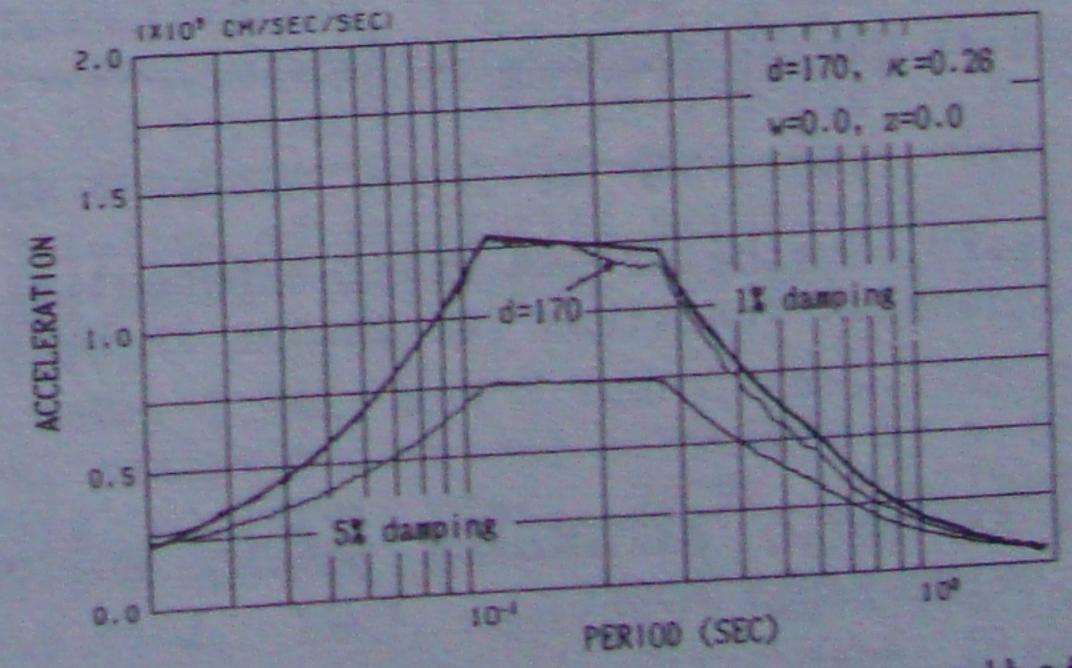
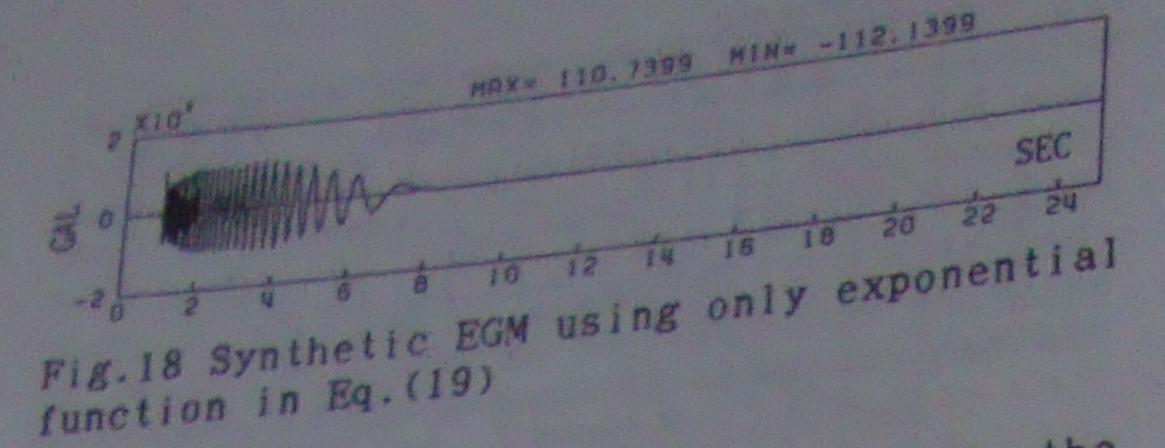


Fig.17 DRS(1) and RSS(1) due to synthetic EGM shown in Fig.18



through some numerical experiments and the interpolation of above results. Time history applied to these coefficients for synthetic EGM is shown in Fig. 18, where the wave form is not so familiar as the one for ordinary recorded EGMs. In order to modify this ill-shaped wave form of the resulted synthetic EGM, coefficients "w" and "z" are considered. These "w" and "z" determine the lower period boundary in period range more affected by randomness function for phases in Eq.19 on RSS(1). If "w" and "z" are adopted as in Fig. 19, this boundary is about 0.15 second. Time history of synthetic EGM using the above values of coefficients is shown in Fig. 20 which may appear to be similar wave form with the recorded EGMs.

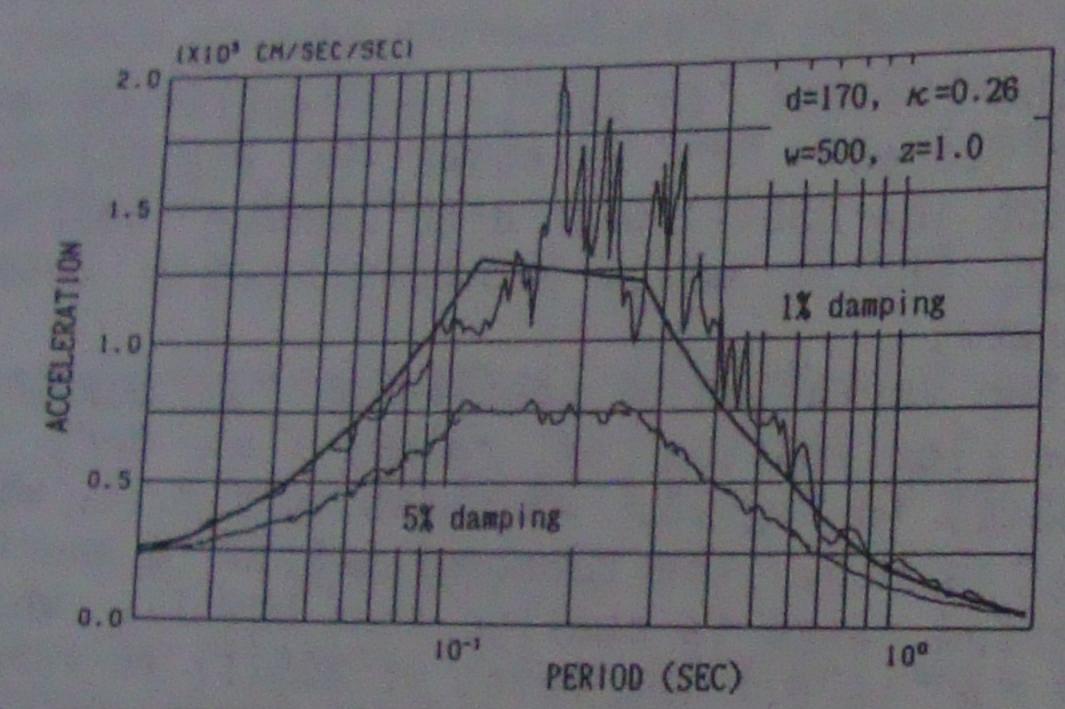


Fig.19 DRSs and RSSs due to final obtained synthetic EGM shown in Fig.20

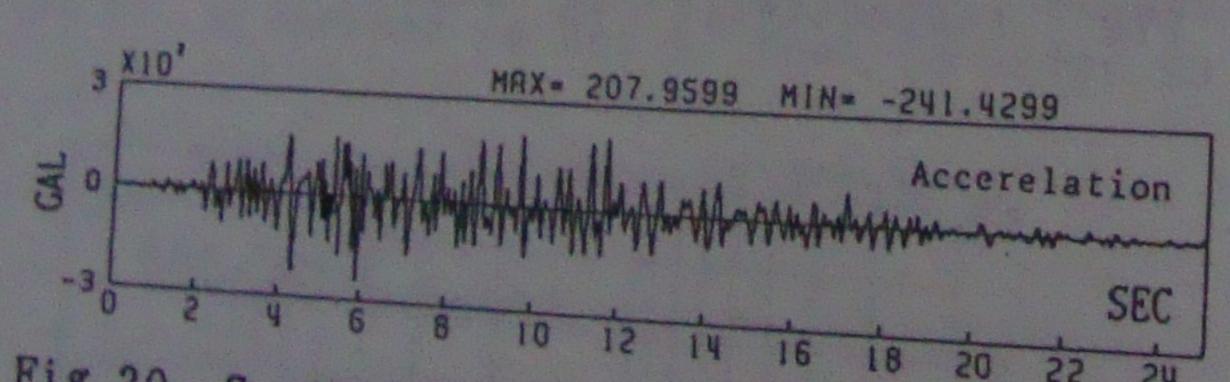


Fig. 20 Synthetic EGM using exponential and randomness functions for phases in Eq. (19)

10 SYNTHETIC EARTHQUAKE GROUND MOTIONS
USING FAULT MODEL

The characteristics of EGMs within the shorter periods than 2-3 seconds have been investigated with better accuracy on the

basis of limited recorded strong EGMs various point of views. However, the from rties of EGMs with longer periods and open cially of the surface waves caused by especially the great mentioned studies might not be adequately designing structures with longer period designing structures with longer period for tunately, it is possible by the seriod logical investigations to evaluate surface wave motions derived from the the consideration of geological structures and the consideration of geological structures and the structures are though which waves propagate.

through which
In Japan, a great earthquake with
tude of 8.0 class has been anticipated
Tokai district at the distance of look
km in the south-west from Tokyo. Fig.20
shows the assumed fault model of Tokai
earthquake by the dislocation of 4 Tokai
and the rise-up time of 4 sec. Using these
parameters, the surface wave motions
model with the Lamb's theory for surface
wave propagation. As for the body-wave
motions by S-wave, the synthetic EGM approperiod range was generated by the procedure

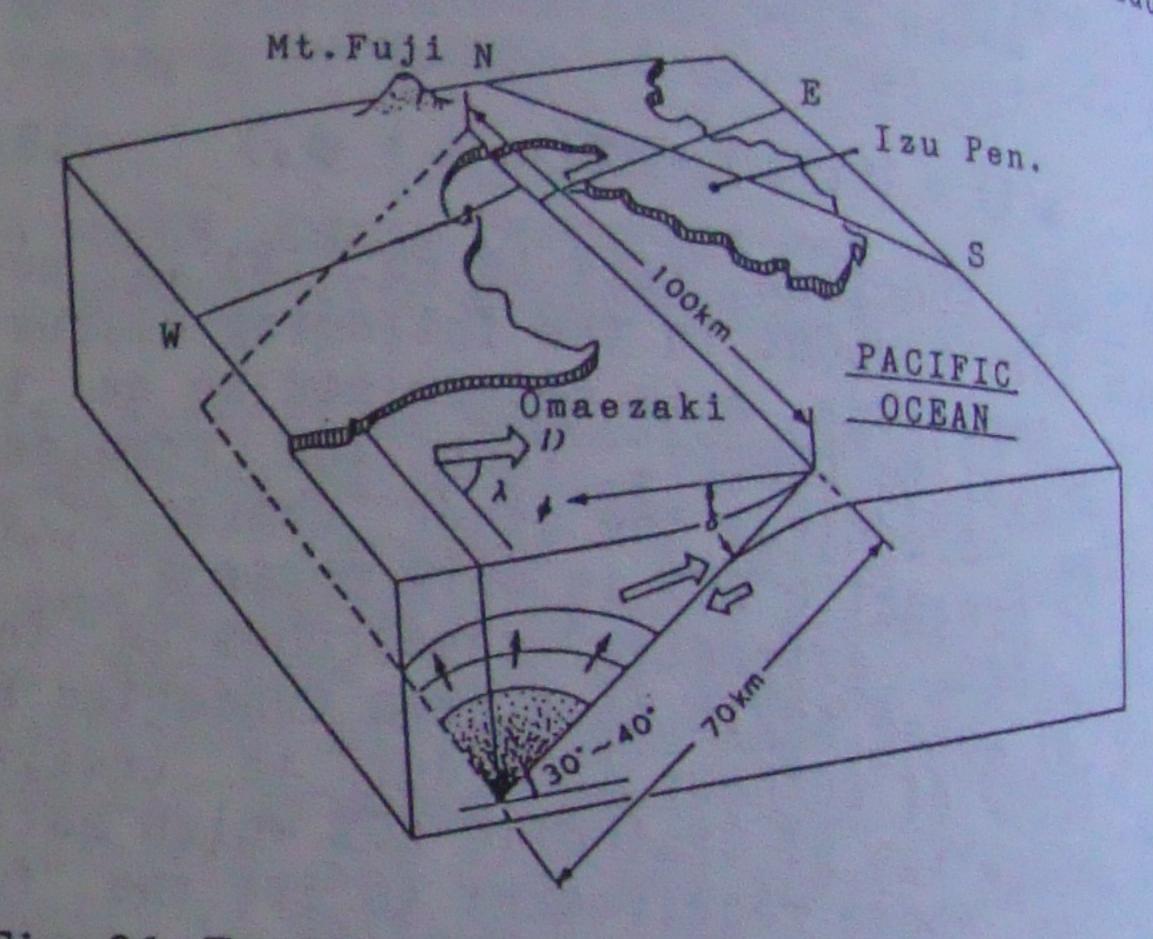


Fig. 21 Fault model of Tokai earthquake

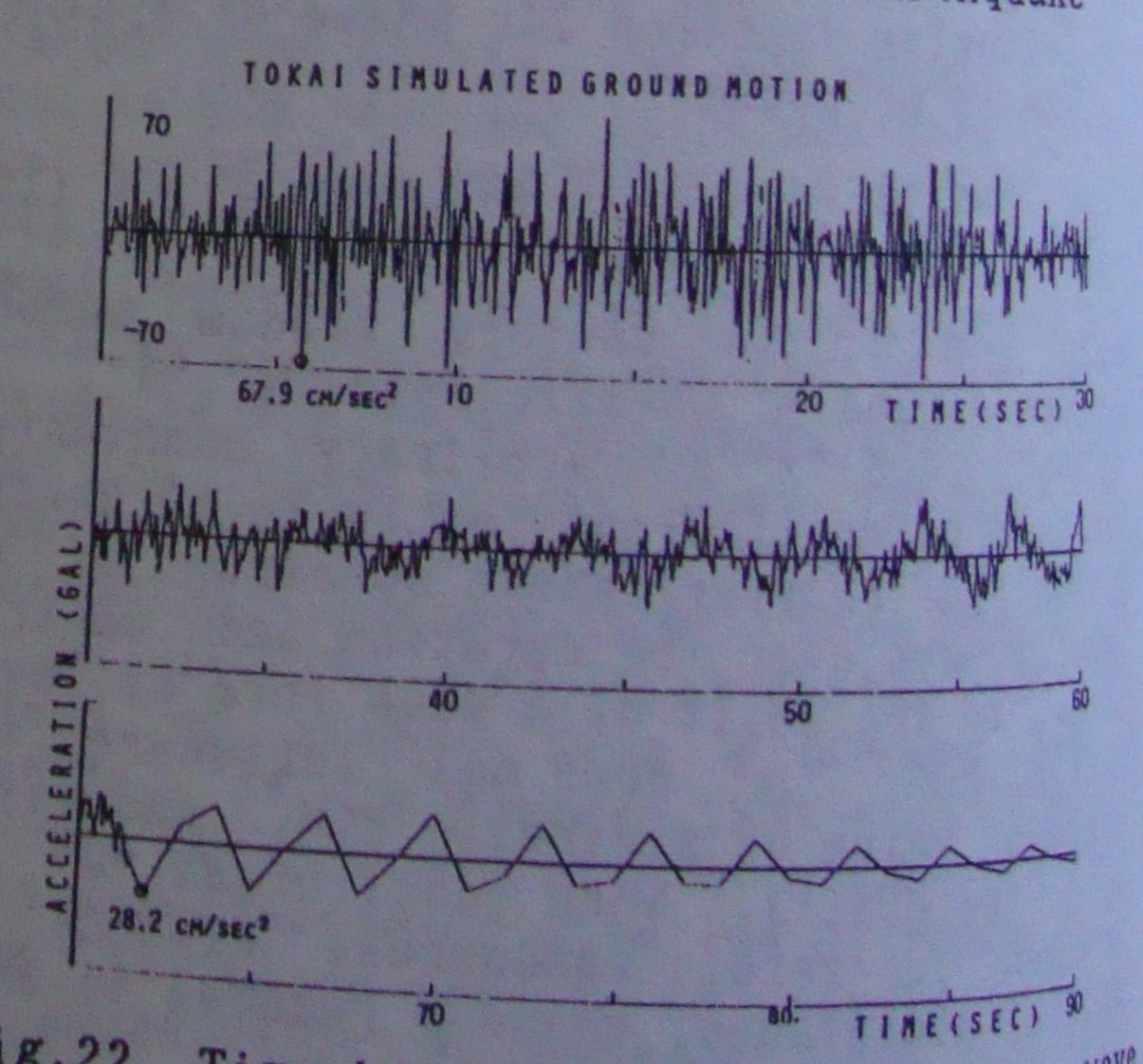


Fig. 22 Time history combined surface wave by seismological approach with body wave

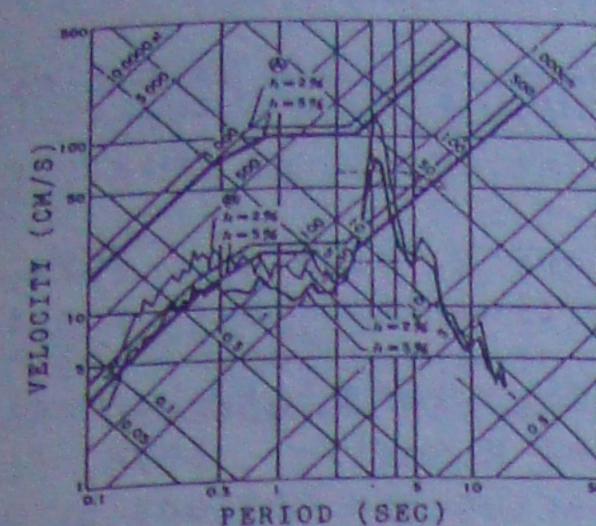


Fig. 23 Comparison between Japanese seismic code and response spectra of synthetic EGM code and shown in Fig. 22

introduced in Sec. 7. The time history combined the surface wave with body wave considering the arrival time of each wave is shown in Fig. 22. In this figure the former and the latter parts of the motion show the body and surface wave, respectively. The response spectra subjected to the motion are shown in Fig. 23 in comparison with acceleration values equivalent to the design base shear coefficients illustrated in solid linear lines. The pair of upper linear lines ( ( ) indicate response values equivalent to the ultimate seismic capacities required by National Building Code in Japan for steel structures (thin line) and for the other structures (thick line), while lower pair (B) are for elastic design. From these comparisons, it was pointed out that the surface wave due to Tokai earthquake may excite as large response nearly close to the ultimate state.

As is introduced, the synthetic EGM combined the theoretical approach by seismological way with the sinusoidal superposition procedure may be quite useful for practical engineering design purposes.

### 11 CONCLUSIONS

The procedure by the sinusoidal superposition method to generate synthetic EGMs may be regarded to be established at least in the domain of the linear response. Yet the following future tasks in this field may be pointed out.

1) Nonstationality property in frequency domain should be extensively investigated for practical use.

2) Research on the procedure to generate synthetic EGMs by establishing fault models which can be compatible with recommoders which can be compatible with recommoder to EGMs in shorter period range should be more encouraged in order to be served for practical earthquake resistant design of the critical structures. This kind of the critical structures to the one on the

longer period range of design EGMs, which are critical factor for earthquake resistant design of structures with longer periods such as high-rise buildings and suspension bridges.

3) Multi-components of design EGMs may be indispensable in very near future. Therefore the extensive research on this field should be urged.

#### REFERENCES

- 1. G.W. Housner, 1947, Characteristics of strong motion earthquakes, Bull. Seism. Soc. Am., Vol. 37, Nol: 19-31
- 2. E.Rosenblueth, 1956, Some applications of Probability theory in aseismic design, Proc. 1WCEE, Berkeley: 1-18
- 3. G.W. Housner, 1959, Behavior of structures during earthquakes, J. Engng Mech. Div., ASCE, Vol.85: 109-129
- 4. N.Bycroft, 1960, White noise representation of earthquakes, J. Engng Mech. Div., ASCE, Vol.86, No.EM2: 1-16
- 5. V.V.Bolotin, 1960, Statistical theory of the aseismic design of structures, Proc. 2WCEE, Vol.2, Tokyo: 1365-1374
- 6. M.F.Barstein, 1960, Application of Probability methods for design the effect of seismic forces on engineering structures, Proc. 2WCEE, Vol.2, Tokyo: 1467-1481
- 7. H.Tajimi, 1960, A statistical method of determining the maximum response of a building structure during an earthquake, Proc. 2WCEE, Vol.2, Tokyo: 781-797
- 8. J.L.Bogdanoff, J.E.Goldberg and M.C. Bernard, 1961, Response of a simple structures to a random earthquake-type disturbance, Bull. Seism. Soc. Am., Vol.51, No.2: 293-310
- 9. E.Rosenblueth and J.I.Bustamante, 1962, Distribution of structural response to earthquakes, J. Engng Mech. Div., ASCE, Vol. 88, No. EM3: 75-106
- 10.Y.K.Lin, 1963, Application of nonstationary shot noise in the study of system response to a class of nonstationary excitations, J. Applied Mech., Vol.30, Series E, No.4: 555-558
- 11.G.W. Housner and P.C. Jennings, 1964, Generation of artificial earthquakes, J. Engng Mech. Div., ASCE, Vol. 90, No. EM1:
- 12.T.Naka, T.Kato and M.Yuasa, 1965, Dynamical analysis of steel structure by elecical analogy, Proc. 3WCEE, Auckland, troic analogy, Proc. 3WCEE, Auckland,
- 13.H.S.Ward, 1965, Analog simulation of earthquake motions, J. Engng Mech. Div., ASCE, Vol. 91, No. EM5: 173-190
- 14.M. Shinozuka and Y. Sato, 1967, Simulation of nonstationary Random process, J. Engng

Mech. Div., ASCE, Vol. 93, No. EM1: 11-40 15.P.C.Jennings, G.W.Housner and N.C.Tsai, 1968, Simulated earthquake motions," EERL

16.M. Amin and A.H.S. Ang, 1968, Nonstationary stochastic model of earthquake motions, J. Engng Mech. Div., ASCE, Vol.94,

17.K. Toki, 1968, Simulation of earthquake motion and its application, Bull. Disaster Prevention Res. Inst., Kyoto Univ.,

Vol.11-A: 291-303(in Japanese)

18. J. Penzien and S.C. Liu, 1969, Nondeterministic analysis of nonlinear structures subjected to earthquake excitations, Proc. 4WCEE, Santiago, Vol.1: 114-129

19. H. Goto and K. Toki, 1969, Structural response to nonstationary random excitation, Proc. 4WCEE, Santiago, Vol.1: 130-

20.H.Goto and H.Kameda, 1969, Statistical inference of the future earthquake ground motion, Proc. 4WCEE, Santiago, Vol.1: 39-54

21.R.N. Iyengar and K.T.S.R. Iyengar, 1969, A nonstationary random process model for earthquake accelerograms, Bull. Seism. Soc. Am., Vol.59, No.3: 1163-1188

22.R.Levy, F.Kozin and R.B.B.Moorman, 1971, Random processes for earthquake simulation, J. Engng Mech. Div., ASCE, Vol. 97, No.EM2: 495-517

23.M. Watabe, 1969, Simulated earthquake acceleration record for dynamic analysis of structures, Bull. Int. Inst. Seism. Earthqu. Engng, Vol.6: 95-102

24. P. Ruiz and J. Penzien, 1971, Stochastic seismic response of structures, J. Engng Mech. Div., ASCE, Vol.97, No.ST4: 441-456

25.M.Shinozuka and C.M.Jan, 1971, Simulation of multi-variate and multi-dimensional random process II, Technical Report 12, Columbia Univ.

26.N.C.Tsai, 1972, Spectrum-compatible motions for design purposes, J. Engng Mech. Div., ASCE, Vol.98, No.EM2: 345-356

27.P.C.Rizzo, D.E.Shaw and S.J.Jarecki, 1973, Development of real/synthetic time histories to match smooth design spectra, Proc. 2nd Int. Conf. on SMiRT, K1/5,

28.G.R.Saragoni and G.C.Hart, 1974, Simulation of artificial earthquakes, Earthqu. Eng. Struct. Dyn., Vol.2: 249-267

29. J. Penzien and M. Watabe, 1975, Characteristics of 3-dimensional earthquake ground motions, Eartqu. Dyn., Vol.3: 365-373

30.S.Levy and J.P.D.Wilkinson, 1976, Generation of artificial time-histories, rich in all frequencies, from given response spectra, Nuclear Engng and Design, Vol.38

: 241-251

31.E.H. Vanmarcke and D.A. Gasparini,
Simulated earthquake ground
Proc. 4th Int. Conf. on SMiRT, RI/9 one
Proc. 4th Int. Conf. on SMiRT, RI/9 one
Proc. 4th Int. Conf. on SMiRT, RI/9 one

Francisco
Francisco
32.M. Hoshiya and R. Isoshima, 1978, Simula nonstation M. Hoshiya and dimensional nonstational tion of multi-dimensional nonstational proc. Simula Proc. Proc. earthquake acceleration, Proc. Proc. JSCE, No. 269: 41-52 (in Japanese) JSCE,

No.269: 41

No.269: 41

33.H. Kameda and M. Sugito, 1978, Prediction

atrong earthquake motions by evolution of strong earthquake motions by evolution of strong earthquake motions by evolution process model, Proc. 5th Japan n nary process model, Proc. 5th Japan Rar.

Engng Sympo., Tokyo: 41-48

thqu. Engile 2, and P.N.Rao, 1979, General 34.R.N. Iyengar and P.N.Rao, 1979, General accel-R.N. Iyengar tion of spectrum compatible acceleroge Earthqu. Eng. Struct. Dyn., Vol. rams, Earthqu. Eng. Struct. Dyn., Vol.7.

35.T. Kubo and J. Penzien, 1979, Simulation of three-dimensional strong ground mo. tions along principal axes, San Fernando Farthqu. Eng. Struct Earthquake, Earthqu. Eng. Struct. Dyn.,

36. Y. Ohsaki, 1979, On the significance of phase content in earthquake ground no. tions, Eartqu. Eng. Struct. Dyn., Vol.7:

37. M. Watabe, R. Iwasaki, M. Tohdo and I.Ohkawa, 1980, Simulation of three. dimensional earthquake ground motions along principal axes, Proc. 7WCEE, Istanbul, Vol.2: 287-294

38.U.S.Regulatory Commission, 1973, Design response spectra for seismic design of nucler power plant, Regulatory Guide 1.60

39. T. Hisada, Y. Ohsaki, M. Watabe and T. Ohta, 1978, Design spectra for stiff structures on rock, 2nd Int. Conf. on Microzonation, San Francisco, Vol.3: 1187-1198

40. Y. Ohsaki, M. Watabe and M. Tohdo, 1980, Analyses on seismic ground motion parameters including vertical components, Proc. 7WCEE, Istanbul, Vol.2: 97-104